This LP LVL guide (formerly Gang-Lam LVL) must be used in conjunction with
the Engineered Wood Product Guide.

## LP LVL $2650 \mathrm{~F}_{\mathrm{b}}-1.9 \mathrm{E}$ Technical Guide

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General Information

## Product Specifications \& Design Values

## DESIGN VALUES (ALLOWABLE STRESS DESIGN-PSI)

| Grade | BENDING <br> $F_{b} *$ | MOE <br> $\left(\times 10^{6}\right)$ | COMPRESSION <br> $F_{\mathrm{c}}$ <br> (parallel to grain) | COMPRESSION <br> $F_{c p}$ <br> (perpendicular to grain) | SHEAR <br> $F_{v}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $2650 \mathrm{~F}_{\mathrm{b}}-1.9 \mathrm{E}$ | 2650 | 1.9 | 2350 | 750 | 285 |

## NOTES:

* $F_{b}$ is for 12 " depth (d).

For depths greater than $12^{\prime \prime}$, adjust $\mathrm{F}_{\mathrm{b}}$ by $(12 / \mathrm{d})^{1 / 7}$.
For depths less than 12 ", adjust $F_{b}$ by $(12 / d)^{1 / 9}$.
For depths less than $5-1 / 2^{\prime \prime}$, adjust $F_{b}$ by 1.09 .

The values above are valid for the following LVL species:
LVL - Southern Yellow Pine
LVL W - Douglas Fir-Larch or Western Hemlock, separately or mixed
LVL L - Lodgepole Pine
LVL LW - Lodgepole Pine and Douglas Fir mixed

The values above are for normal load duration ( $100 \%$ ). Bending $\left(F_{b}\right)$, Compression Parallel-to-Grain $\left(F_{c}\right)$ and Shear $\left(F_{v}\right)$ may be adjusted according to code. MOE (E) and Compression Perpendicular-to-Grain ( $\mathrm{F}_{\mathrm{Cp}}$ ) shall NOT be adjusted.

SECTION PROPERTIES AND ALLOWABLE CAPACITIES

| Depth | Weight ( $\mathrm{lb} / \mathrm{ft}$ ) |  |  | Allowable Moment (lb-ft) |  |  | Allowable Shear (b) |  |  | Moment of Inertia (in ${ }^{4}$ ) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1-3/4" | $3-1 / 2^{1}$ | 5-1/4" | 1-3/4" | $3-1 / 2^{1}$ | 5-1/4" | 1-3/4" | $3-1 / 2^{\prime \prime}$ | 5-1/4" | 1-3/4" | $3-1 / 2^{1}$ | 5-1/4" |
| 7-1/4" | 3.6 | 7.3 | 10.9 | 3580 | 7161 | 10741 | 2411 | 4821 | 7232 | 55 | 111 | 167 |
| 9-1/2" | 4.8 | 9.5 | 14.3 | 5966 | 11932 | 17898 | 3159 | 6318 | 9476 | 125 | 250 | 375 |
| 11-7/8" | 5.9 | 11.9 | 17.8 | 9093 | 18187 | 27280 | 3948 | 7897 | 11845 | 244 | 488 | 733 |
| $14^{\prime \prime}$ | 7.0 | 14.0 | 21.0 | 12349 | 24699 | 37048 | 4655 | 9310 | 13965 | 400 | 800 | 1201 |
| $16^{\prime \prime}$ | 8.0 | 16.0 | 24.0 | 15825 | 31650 | 47475 | 5320 | 10640 | 15960 | 597 | 1195 | 1792 |
| $18^{\prime \prime}$ | 9.0 | 18.0 | 27.0 | 19694 | 39389 | 59083 | 5985 | 11970 | 17955 | 851 | 1701 | 2552 |

## MODIFICATION FACTORS:

The Allowable Moment (M) and Shear (V) above are for normal load duration (100\%) and may be adjusted according to code.

## FASTENER VALUES:

Refer to the current ICC ES evaluation report (ICC-ES Report ESR-1254) for species-specific information on the equivalent specific gravity for design of nail and bolt connections. ICC ES evaluation reports can be obtained online at www.iccsafe.org


## BEARING LENGTH AND MAXIMUM REACTION CHART

How to use bearing charts:

1. Determine the thickness required for the LP LVL beam and calculate the maximum reaction.
2. Select the appropriate table for 1 -ply ( $1-3 / 4^{\prime \prime}$ ), 2-ply ( $3-1 / 2^{\prime \prime}$ ) or 3 -ply ( $5-1 / 4^{\prime \prime}$ ).
3. Select a bearing length with a maximum reaction that meets or exceeds your calculated value.
4. Make sure the support is structurally adequate to carry the reaction.

Example: 3-1/2" LP LVL with a reaction of 9,500 Ibs.
Solution: Select a 4" bearing length with a maximum reaction of $10,500 \mathrm{lbs}$.

## MAXIMUM REACTION (LBS.)

| Bearing Length (in) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Width | 1-1/2 | 2 | 2-1/2 | 3 | 3-1/2 | 4 | 4-1/2 | 5 | 5-1/2 | 6 | 6-1/2 | 7 | 7-1/2 | 8 | 8-1/2 | 9 | 9-1/2 | 10 | 10-1/2 | 11 | 11-1/2 | 12 |
| 1-3/4" | 1968 | 2625 | 3281 | 3937 | 4593 | 5250 | 5906 | 6562 | 7218 | 7875 | 8531 | 9187 | 9843 | 10500 | 11156 | 11812 | 12468 | 13125 | 13781 | 14437 | 15093 | 15750 |
| 3-1/2" | 3937 | 5250 | 6562 | 7875 | 9187 | 10500 | 11812 | 13125 | 14437 | 15750 | 17062 | 18375 | 19687 | 21000 | 22312 | 23625 | 24937 | 26250 | 27562 | 28875 | 30187 | 31500 |
| 5-1/4" | 5906 | 7875 | 9843 | 11812 | 13781 | 15750 | 17718 | 19687 | 21656 | 23625 | 25593 | 27562 | 29531 | 31500 | 33468 | 35437 | 37406 | 39375 | 41343 | 43312 | 45281 | 47250 |

## NOTES:

1. Tabulated values are based on the allowable compression stress, perpendicular to grain, of the LVL. This is suitable for beams bearing on steel or the end grain of studs.
2. Make sure the support is structurally adequate to carry the reaction. Compressive strength parallel to grain of studs may require more studs than the bearing length above indicates.
3. For beams bearing on wood plates, the required bearing length will increase based on the bearing strength (compression perpendicular to grain) of the species and grade used for the plate material.
4. Verify local code requirements concerning minimum bearing.

## Floor Beam Quick Reference Tables

## To use these charts:

1. Select the correct table for the beam application you need.
2. Choose the required beam span in the left column.
3. Select the span carried on the top line.
4. Read the beam size or choice of beam sizes from table.

Example: A $14^{\prime}-0$ " span beam carries $15^{\prime}-0{ }^{\prime \prime}$ simple span joists on each side.
Solution: Using table below, $30^{\prime}-0^{\prime \prime}$ span carried, select either $3-1 / 2^{\prime \prime} \times 14^{\prime \prime}$ or $5-1 / 4^{\prime \prime} \times 11-7 / 8^{\prime \prime}$.


FOR FLOOR JOISTS THAT ARE CONTINUOUS (ONE PIECE) (40 PSF LIVE, 15 PSF DEAD, 100\%)

| Beam Span (tt) | Beam Width | Span Carried By Beam (ft) |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 20 | 22 | 24 | 26 | 28 | 30 | 32 | 34 | 36 | 38 | 40 |
| 8 | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" |
| 10 | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | $9-1 / 4^{\prime \prime}$ | 9-1/4" | 9-1/4" | $9-1 / 4^{\prime \prime}$ | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 12 | $3-1 / 2^{\prime \prime}$ | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 14 " | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
| 14 | 3-1/2" | 11-7/8" | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16 "$ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ |
|  | $5-1 / 4^{\prime \prime}$ | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14 | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ |
| 16 | 3-1/2" | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $16^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | 18 " | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ |
|  | 5-1/4" | 11-7/8" | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $16^{\prime \prime}$ |
| 18 | 3-1/2" | $16{ }^{\prime \prime}$ | 16 " | 18 " | $18{ }^{\prime \prime}$ | 18 " | 18 " | - | - | - | - | - |
|  | 5-1/4" | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14 "$ | $14{ }^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |
| 20 | 3-1/2" | 18" | 18" | 18 " | - | - | - | - | - | - | - | - |
|  | $5-1 / 4^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $18^{\prime \prime}$ | $18{ }^{\prime \prime}$ | 18" | $18{ }^{\prime \prime}$ | 18" |

For floor joists that are continuous over the beam:

- Floor joist spans are approximately equal on each side of beam.
- Beam Span is valid for simple span beams and continuous, equal span beams.
- 3 " bearing length is required at end supports.
- $6^{\prime \prime}$ bearing length is required at interior supports EXCEPT 7-1/2" bearing is required where bold.


FOR FLOOR JOISTS THAT ARE NOT CONTINUOUS (40 PSF LIVE, 15 PSF DEAD, 100\%)

| Beam Span (tt) | Beam Width | Span Carried By Beam (tt) |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 20 | 22 | 24 | 26 | 28 | 30 | 32 | 34 | 36 | 38 | 40 |
| 8 | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 10 | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | $9-1 / 4^{\prime \prime}$ | $9-1 / 4^{\prime \prime}$ | $9-1 / 4^{\prime \prime}$ | 9-1/4" | 9-1/4" |
| 12 | 3-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | $14{ }^{\prime \prime}$ |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2 ${ }^{1}$ | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
| 14 | $3-1 / 2^{\prime \prime}$ | 11-1/4" | 11-7/8" | 11-7/8" | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $16{ }^{\prime \prime}$ |
|  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" |
| 16 | $3-1 / 2^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $18{ }^{\prime \prime}$ |
|  | 5-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14 "$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ |
| 18 | 3-1/2" | $16^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16^{\prime \prime}$ | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | - |
|  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |
| 20 | 3-1/2" | 16 " | 16 " | 18 " | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | 18" | - |  | - | - |  |
|  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $18{ }^{\prime \prime}$ |

For floor joists that are NOT continuous over the beam:

- Floor joists either lap or butt on top of beam, or frame into beam with hangers.
- Beam Span is valid for simple span beams and continuous, equal span beams.
- 3 " bearing length is required at end supports.
- $6^{\prime \prime}$ bearing length is required at interior supports EXCEPT 7-1/2" bearing is required where bold.


## GENERAL NOTES:

1. Deflection criteria for quick reference tables: roofs: $\mathrm{L} / 240$ deflection limit for live load and $\mathrm{L} / 180$ for total load floors: L/360 deflection limit for live load and L/240 for total load.


Blocking or other approved method of restraint required
2. A deflection criteria of $\mathrm{L} / 240$ indicates the maximum deflection allowed for a $10^{\prime}-0^{\prime \prime}$ span beam is $10 \times 12 / 240=1 / 2^{\prime \prime}$.
3. Deeper beams or an additional ply will increase beam stiffness and reduce deflection.
4. Beam Width can be either a single piece of LVL or built up from individual plies of LVL that are nailed and/or bolted together. Refer to page 13 of the Engineered Wood Product Guide for connection details.
5. Floor live loads have been reduced in accordance with the 2000/2003 IBC (ICC) section 1607.9.2, 1997 UBC (ICBO) section 1607.5, 1999 NBC (BOCA) section 1606.7 and 1999 SBC (SBCCI) section 1604.2.

## Combined Header Quick Reference Tables

For combined roof and floor loads：
－For simple span headers only（headers with a support at each end）．
－Roof loads include a $2^{2}$ overhang．
－Loads include 100 plf wall load．
－Interior support at mid－span of floor joists is required．
－Minimum bearing length is 3 ＂， $4-1 / 2^{\prime \prime}$ bearing length is required where bold
－Read notes and instructions for quick reference tables on page 3.


|  | Beam | Beam Width |  |  |  |  | rried By B |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Span（ft） | Beam Wath | 20 | 22 | 24 | 26 | 28 | 30 | 32 | 34 | 36 | 38 | 40 |
| 응응 |  | $3-1 / 2^{\prime \prime}$ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
| 品辰 | 6 | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
| ¢ 之＜ |  | $3-1 / 2^{\prime \prime}$ | 7－1／4＂ | 7－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ |
| ¢ | 8 | $5-1 / 4^{\prime \prime}$ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 9－1／4＂ |
| 山i̊ ${ }_{\text {¢ }}^{\text {® }}$ | 10 | 3－1／2＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ |
| 二－山 |  | 5－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ |
| 以 |  | $3-1 / 2^{\prime \prime}$ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 14 ＂ | $14{ }^{\prime \prime}$ |
| べひぐ | 12 | 5－1／4＂ | 9－1／4＂ | 9－1／2 ${ }^{\text {＂}}$ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ |
|  |  | 3－1／2＂ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $16^{\prime \prime}$ | 16 ＂ | $16{ }^{\prime \prime}$ | $16 "$ | 16 ＂ |
|  | 14 | 5－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | 11－7／8＂ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 14 ＂ | $14{ }^{\prime \prime}$ |
|  |  | $3-1 / 2^{\prime \prime}$ | 14 ＂ | 16 ＂ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | 18＂ | 18 ＂ | 18＂ | 18＂ | － |
|  |  | $5-1 / 4^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 16 ＂ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |


|  | Beam |  |  |  |  |  | aried By |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Span（ft） | Beam Width | 20 | 22 | 24 | 26 | 28 | 30 | 32 | 34 | 36 | 38 | 40 |
|  |  | 3－1／2＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
| － | 6 | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
| 岗 |  | $3-1 / 2^{\prime \prime}$ | 7－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ |
| － | 8 | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ |
| いろう |  | 3－1／2＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ |
|  | 10 | 5－1／4＂ | $9-1 / 4^{\prime \prime}$ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 9－1／2＂ |
| むこ |  | 3－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | $14 "$ | $14 "$ | 14 ＂ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ |
| $\stackrel{\sim}{\sim}$ | 12 | 5－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ |
| $\ddot{\square}$ | 14 | $3-1 / 2^{\prime \prime}$ | $14^{\prime \prime}$ | 14 ＂ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 18＂ |
|  | 14 | 5－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14 "$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ |
|  |  | 3－1／2＂ | $16^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | 18＂ | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | － | － |
|  | 16 | 5－1／4＂ | $14 "$ | $14 "$ | 14 ＂ | $14^{\prime \prime}$ | 14 ＂ | $14 "$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |


|  | Beam | Beam Width |  |  |  |  | rried By |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Span（ft） | Beam Wiath | 20 | 22 | 24 | 26 | 28 | 30 | 32 | 34 | 36 | 38 | 40 |
|  |  | 3－1／2＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
| 崰 | 6 | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
| ¢ |  | 3－1／2＂ | 7－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ |
| $\xrightarrow{3}$ | 8 | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ |
| いでき |  | $3-1 / 2^{\prime \prime}$ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ |
| 攷耑 | 10 | 5－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 9－1／2 ${ }^{1}$ | 11－1／4＂ |
| ぶ궁 |  | $3-1 / 2^{\prime \prime}$ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | $14^{\prime \prime}$ | 14 ＂ | $14 "$ | $14 "$ | $14{ }^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |
| －$\stackrel{\text { ¢ }}{0}$ |  | 5－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ |
| $\ddot{\square}$ |  | 3－1／2＂ | 14 ＂ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14 "$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 18＂ | 18＂ | 18＂ |
|  | 14 | 5－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ |
|  |  | $3-1 / 2^{\prime \prime}$ | $16^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16^{\prime \prime}$ | 18＂ | 18＂ | $18{ }^{\prime \prime}$ | 18 ＂ | － | － | － | － |
|  |  | 5－1／4＂ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |


|  | Beam | Beam Width |  |  |  |  | ried By B |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Span（ft） | Beam war | 20 | 22 | 24 | 26 | 28 | 30 | 32 | 34 | 36 | 38 | 40 |
| ¢ |  | 3－1／2＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 9－1／4＂ | 9－1／4＂ |
| － | 6 | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
| ¢ ๑ |  | 3－1／2＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ |
| － | 8 | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ |
| 山己ご |  | $3-1 / 2^{\prime \prime}$ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | $14 "$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ |
| $\geq \circ \circ \frac{1}{3}$ | 10 | 5－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ |
| ¢ $\begin{gathered}\text { ¢ }\end{gathered}$ |  | $3-1 / 2^{\prime \prime}$ | 11－7／8＂ | 11－7／8＂ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 16 ＂ | 16 ＂ |
| 아 | 12 | 5－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | 11－7／8＂ | $14^{\prime \prime}$ | $14^{\prime \prime}$ |
| 붕 |  | 3－1／2＂ | 14 ＂ | 14 ＂ | 16 ＂ | 16 ＂ | 16 ＂ | 16 ＂ | 18＂ | 18＂ | 18＂ | 18＂ | － |
| 은 | 14 | 5－1／4＂ | 11－7／8＂ | 11－7／8＂ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 14 ＂ | $14 "$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |
|  |  | 3－1／2＂ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | 18＂ | 18 ＂ | 18 ＂ | － | － | － | － | － | － |
|  | 16 | 5－1／4＂ | $14^{\prime \prime}$ | $14 "$ | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $18^{\prime \prime}$ | $18^{\prime \prime}$ | $18{ }^{\prime \prime}$ |

## Roof Header Quick Reference Tables

For roof loads：
－For simple span headers only（headers with a support at each end）．
－Roof loads include a $2^{\prime}$ overhang．
－Minimum bearing length is $3^{\prime \prime}, 4-1 / 2^{\prime \prime}$ bearing length is required where bold．
－Read notes and instructions for quick reference tables on page 3.


|  | Beam | Beam Width |  |  |  |  | arried By |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Span（ft） | Beam Wan | 20 | 22 | 24 | 26 | 28 | 30 | 32 | 34 | 36 | 38 | 40 |
| $\begin{aligned} & 3 \\ & 0 \end{aligned}$ |  | 3－1／2＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
|  | 8 | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
| $\geq{ }^{\text {² }}$ | 10 | 3－1／2＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ |
| 号安。 | 10 | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
| の ${ }^{\text {¢ }}$ |  | 3－1／2＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ |
| へ ¢ ¢ | 12 | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ |
|  |  | 3－1／2＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ |
| $\bigcirc$ | 14 | 5－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 9－1／2＂ | 11－1／4＂ |
|  |  | 3－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | 11－7／8＂ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 14 ＂ |
|  | 16 | 5－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ |
|  |  | $3-1 / 2^{\prime \prime}$ | 11－7／8＂ | 11－7／8＂ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |
|  | 18 | 5－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | 11－7／8＂ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ |


| BeamSpan（ft） |  | Beam Width | Span Carried By Beam（ft） |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 20 | 22 | 24 | 26 | 28 | 30 | 32 | 34 | 36 | 38 | 40 |
|  | 8 |  | 3－1／2＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
|  |  | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
|  | 10 | $3-1 / 2^{\prime \prime}$ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ |
|  |  | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
|  | 12 | $3-1 / 2^{\prime \prime}$ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ |
|  |  | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ |
|  | 14 | 3－1／2＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | 14 ＂ | $14 "$ |
|  |  | 5－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ |
|  | 16 | 3－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | 14 ＂ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $16^{\prime \prime}$ |
|  |  | 5－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ |
|  | 18 | 3－1／2＂ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 14 ＂ | 14 ＂ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 16 ＂ | $16^{\prime \prime}$ | 16 ＂ | $16 "$ | $16 "$ |
|  |  | 5－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | 11－7／8＂ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14 "$ | $14 "$ | $14 "$ |


|  | $\begin{gathered} \text { Beam } \\ \text { Span (ft) } \end{gathered}$ | Beam Width | Span Carried By Beam（tt） |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | 20 | 22 | 24 | 26 | 28 | 30 | 32 | 34 | 36 | 38 | 40 |
|  | 8 | 3－1／2＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
|  |  | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
|  | 10 | 3－1／2＂ | 7－1／4＂ | 7－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ |
|  |  | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 9－1／4＂ | $9-1 / 4^{\prime \prime}$ |
|  | 12 | $3-1 / 2^{\prime \prime}$ | $9-1 / 4^{\prime \prime}$ | $9-1 / 4^{\prime \prime}$ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ |
|  |  | 5－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ |
|  | 14 | $3-1 / 2^{\prime \prime}$ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ |
|  |  | 5－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ |
|  | 16 | $3-1 / 2^{\prime \prime}$ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16{ }^{\prime \prime}$ |
|  |  | 5－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | 11－7／8＂ | $14^{\prime \prime}$ |
|  | 18 | $3-1 / 2^{\prime \prime}$ | $14 "$ | $14{ }^{\prime \prime}$ | 14 ＂ | $14{ }^{\prime \prime}$ | 16 ＂ | $16{ }^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 18 ＂ | 18 ＂ | 18＂ |
|  |  | $5-1 / 4^{\prime \prime}$ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | 11－7／8＂ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | 14 ＂ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ |


| Beam Span（ft） |  | Beam Width | Span Carried By Beam（tt） |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 20 | 22 | 24 | 26 | 28 | 30 | 32 | 34 | 36 | 38 | 40 |
|  | 8 |  | $3-1 / 2^{\prime \prime}$ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ |
|  |  | 5－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ |
|  | 10 | 3－1／2＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ |
|  |  | $5-1 / 4^{\prime \prime}$ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 7－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ |
|  | 12 | 3－1／2＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | 14 ＂ | $14{ }^{\prime \prime}$ |
|  |  | $5-1 / 4^{\prime \prime}$ | $9-1 / 4^{\prime \prime}$ | $9-1 / 4^{\prime \prime}$ | 9－1／4＂ | $9-1 / 4^{\prime \prime}$ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ |
|  | 14 | $3-1 / 2^{\prime \prime}$ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 14 ＂ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ |
|  |  | 5－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ |
|  | 16 | $3-1 / 2^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 14 ＂ | 16 ＂ | $16{ }^{\prime \prime}$ | 16 ＂ | 16 ＂ | 16 ＂ | 18＂ | 18＂ |
|  |  | 5－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－1／4＂ | 11－7／8＂ | 11－7／8＂ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 14 ＂ | $14^{\prime \prime}$ | 14 ＂ |
|  | 18 | $3-1 / 2^{\prime \prime}$ | 14 ＂ | $16^{\prime \prime}$ | 16 ＂ | 16 ＂ | $16{ }^{\prime \prime}$ | 18 ＂ | 18＂ | 18＂ | － | － | － |
|  |  | $5-1 / 4^{\prime \prime}$ | 11－7／8＂ | $14{ }^{\prime \prime}$ | 14 ＂ | 14 ＂ | 14 ＂ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |

## Uniform Floor Load (PLF) Tables

How to use maximum uniform load tables:

1. Select the correct table for the beam application you need.
2. Choose the required beam span in the left column.
3. Select a beam depth from the tables that satisfies both the live and total load plf on the beam.
4. Check the bearing requirements as shown on page 2.

EXAMPLE:
Floor live load 480 PLF, L/360 deflection limit. Floor total load 660 PLF, L/240 deflection limit. Beam span 14'- 0 ".
SOLUTION:
Try 2-ply $1-3 / 4^{\prime \prime} \times 11-7 / 8^{\prime \prime}$, which can carry:

- Live load $2 \times 250=500>480$ PLF $\boldsymbol{V}$ OK
- Total load $2 \times 365=730>660$ PLF $\downarrow$ OK


## ALLOWABLE FLOOR LOADS (PLF) 100\%

| Span (ft) | $1-3 / 4^{\prime \prime} \times 7-1 / 4^{\prime \prime}$ |  |  | $1-3 / 4^{\prime \prime} \times 9-1 / 4^{\prime \prime}$ |  |  | $1-3 / 4^{\prime \prime} \times 9-1 / 2^{\prime \prime}$ |  |  | $1-3 / 4^{\prime \prime} \times 11-1 / 4^{\prime \prime}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Live Load |  | $\begin{aligned} & \text { Total } \\ & \text { Load } \end{aligned}$ | Live Load |  | $\begin{aligned} & \text { Total } \\ & \text { Load } \end{aligned}$ | Live Load |  | Total Load | Live Load |  | Total <br> Load |
|  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  |
| 6 | 543 | 724 | 762 |  |  | 1027 |  |  | 1062 |  |  | 1324 |
| 7 | 342 | 456 | 580 | 710 |  | 848 | 769 |  | 876 |  |  | 1082 |
| 8 | 229 | 305 | 443 | 475 | 634 | 704 | 515 | 687 | 740 | 856 |  | 915 |
| 9 | 160 | 214 | 318 | 334 | 445 | 555 | 362 | 482 | 584 | 601 |  | 792 |
| 10 | 117 | 156 | 231 | 243 | 324 | 449 | 263 | 351 | 472 | 438 | 584 | 651 |
| 11 | 88 | 117 | 172 | 183 | 244 | 361 | 198 | 264 | 389 | 329 | 439 | 537 |
| 12 | 67 | 90 | 132 | 141 | 188 | 277 | 152 | 203 | 300 | 253 | 338 | 450 |
| 13 | 53 | 71 | 103 | 110 | 147 | 217 | 120 | 160 | 235 | 199 | 266 | 383 |
| 14 | 42 | 57 | 81 | 88 | 118 | 172 | 96 | 128 | 187 | 159 | 213 | 313 |
| 15 | 34 | 46 | 65 | 72 | 96 | 139 | 78 | 104 | 151 | 129 | 173 | 254 |
| 16 | - | - | - | 59 | 79 | 114 | 64 | 85 | 124 | 107 | 142 | 208 |
| 17 | - | - | - | 49 | 66 | 94 | 53 | 71 | 102 | 89 | 118 | 172 |
| 18 | - | - | - | 41 | 55 | 78 | 45 | 60 | 85 | 75 | 100 | 144 |
| 19 | - | - | - | 35 | 47 | 66 | 38 | 51 | 72 | 63 | 85 | 122 |
| 20 | - | - | - | 30 | 40 | 56 | 32 | 43 | 61 | 54 | 73 | 103 |
| 21 | - | - | - | - | - | - | - | - | - | 47 | 63 | 89 |
| 22 | - | - | - | - | - | - | - | - | - | 41 | 54 | 76 |
| 23 | - | - | - | - | - | - | - | - | - | 36 | 48 | 66 |
| 24 | - | - | - | - | - | - | - | - | - | 31 | 42 | 57 |


| Span (ft) | $1-3 / 4^{\prime \prime} \times 11-7 / 8^{\prime \prime}$ |  |  | $1-3 / 4^{\prime \prime} \times 14^{\prime \prime}$ |  |  | $1-3 / 4^{\prime \prime} \times 16^{\prime \prime}$ |  |  | $1-3 / 4^{\prime \prime} \times 18^{\prime \prime}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Live Load |  | Total Load | Live Load |  | Total <br> Load | Live Load |  | Total Load | Live Load |  | Total Load |
|  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  |
| 6 |  |  | 1424 |  |  | 1794 |  |  | 2193 |  |  | 2650 |
| 7 |  |  | 1160 |  |  | 1443 |  |  | 1741 |  |  | 2072 |
| 8 |  |  | 978 |  |  | 1207 |  |  | 1442 |  |  | 1700 |
| 9 | 707 |  | 845 |  |  | 1037 |  |  | 1231 |  |  | 1441 |
| 10 | 515 | 687 | 721 | 844 |  | 908 |  |  | 1074 |  |  | 1250 |
| 11 | 387 | 516 | 595 | 634 |  | 808 | 947 |  | 951 |  |  | 1104 |
| 12 | 298 | 397 | 499 | 488 | 651 | 679 | 729 |  | 854 |  |  | 988 |
| 13 | 234 | 312 | 424 | 384 | 512 | 577 | 573 |  | 741 | 817 |  | 894 |
| 14 | 187 | 250 | 365 | 307 | 410 | 497 | 459 | 612 | 637 | 654 |  | 794 |
| 15 | 152 | 203 | 299 | 250 | 333 | 432 | 373 | 498 | 554 | 532 |  | 691 |
| 16 | 125 | 167 | 245 | 206 | 274 | 378 | 307 | 410 | 486 | 438 | 584 | 606 |
| 17 | 104 | 139 | 203 | 171 | 229 | 334 | 256 | 342 | 430 | 365 | 487 | 536 |
| 18 | 88 | 117 | 170 | 144 | 193 | 282 | 216 | 288 | 382 | 307 | 410 | 477 |
| 19 | 75 | 100 | 144 | 123 | 164 | 239 | 183 | 245 | 342 | 261 | 349 | 427 |
| 20 | 64 | 85 | 122 | 105 | 140 | 204 | 157 | 210 | 307 | 224 | 299 | 384 |
| 21 | 55 | 74 | 105 | 91 | 121 | 175 | 136 | 181 | 264 | 193 | 258 | 348 |
| 22 | 48 | 64 | 90 | 79 | 105 | 151 | 118 | 157 | 228 | 168 | 224 | 316 |
| 23 | 42 | 56 | 78 | 69 | 92 | 131 | 103 | 138 | 199 | 147 | 196 | 286 |
| 24 | 37 | 49 | 68 | 61 | 81 | 115 | 91 | 121 | 174 | 129 | 173 | 250 |
| 25 | 32 | 43 | 60 | 54 | 72 | 101 | 80 | 107 | 153 | 114 | 153 | 220 |
| 26 | - | - | - | 48 | 64 | 89 | 71 | 95 | 135 | 102 | 136 | 195 |
| 27 | - | - | - | 42 | 57 | 78 | 64 | 85 | 120 | 91 | 121 | 173 |
| 28 | - | - | - | 38 | 51 | 69 | 57 | 76 | 106 | 81 | 109 | 154 |
| 29 | - | - | - | 34 | 46 | 62 | 51 | 68 | 95 | 73 | 98 | 138 |
| 30 | - | - | - | 31 | 41 | 55 | 46 | 62 | 85 | 66 | 88 | 123 |

## NOTES:

1. Span is defined as center-to-center of bearings and is valid for simple span and equal, multiple span conditions.
2. These loads assume full lateral bracing of the compression edge. Full support is considered to be a maximum unbraced length of 24".
3. The designer must check the Total Load column AND the appropriate Live Load column, either the L/480 or L/360 deflection limit. Live Load values that are blank are governed by Total Load. Do not use a product where designated "-" without further analysis by a professional engineer
4. The Total Load columns are limited to a deflection of $\mathrm{L} / 240$ under Total Load and do not include the effects of long term loading (creep).
5. The Total Load columns have been adjusted to account for the self-weight of the beam.
6. Proper bearing must be provided. Bearing length must be checked for support reactions with the table on page 2.
7. Depths of $16^{\prime \prime}$ and greater should be used with a minimum of two plies unless designed specifically as a single ply of $1-3 / 4^{"}$ with proper lateral bracing spaced at most every 24 " along the length of the beam. (Example: The marriage beam for each half of a manufactured home before the units are joined.)
8. The values in the table are for a single ply of $1-3 / 4^{\prime \prime}$ LVL. Double the values for 2 -ply or $3-1 / 2^{\prime \prime}$ thickness. (Or divide design loads by 2 to use the table directly to verify each ply of a 2 -ply beam.) Triple the values for 3 -ply or $5-1 / 44^{\prime \prime}$ thickness. (Or divide design loads by 3 to use the table directly to verify each ply of a 3 -ply beam.) Quadruple the values for 4 -ply or 7" thickness. (Or divide the design loads by 4 to use the table directly to verify each ply of a 4-ply beam.)
9. Values have NOT been evaluated for vibration.
ACTUAL DEFLECTION

| Span (ft) | $\mathrm{L} / 480$ | $\mathrm{~L} / 360$ | $\mathrm{~L} / 240$ |
| :---: | :---: | :---: | :---: |
| 10 | $1 / 4^{\prime \prime}$ | $5 / 16^{\prime \prime}$ | $1 / 2^{\prime \prime}$ |
| 12 | $5 / 16^{\prime \prime}$ | $3 / 8^{\prime \prime}$ | $5 / 8^{\prime \prime}$ |
| 14 | $3 / 8^{\prime \prime}$ | $7 / 16^{\prime \prime}$ | $11 / 16^{\prime \prime}$ |
| 16 | $3 / 8^{\prime \prime}$ | $9 / 16^{\prime \prime}$ | $13 / 16^{\prime \prime}$ |
| 18 | $7 / 16^{\prime \prime}$ | $5 / 8^{\prime \prime}$ | $7 / 8^{\prime \prime}$ |
| 20 | $1 / 2^{\prime \prime}$ | $11 / 16^{\prime \prime}$ | $1^{\prime \prime}$ |
| 22 | $9 / 16^{\prime \prime}$ | $3 / 4^{\prime \prime}$ | $1-1 / 8^{\prime \prime}$ |
| 24 | $5 / 8^{\prime \prime}$ | $13 / 16^{\prime \prime}$ | $1-3 / 16^{\prime \prime}$ |
| 26 | $5 / 8^{\prime \prime}$ | $7 / 8^{\prime \prime}$ | $1-5 / 16^{\prime \prime}$ |
| 28 | $11 / 16^{\prime \prime}$ | $15 / 16^{\prime \prime}$ | $1-3 / 8^{\prime \prime}$ |
| 30 | $3 / 4^{\prime \prime}$ | 1 " | $1-1 / 2^{\prime \prime}$ |

[^0]
## ALLOWABLE ROOF LOADS (PLF)

| Span (ft) | $1-3 / 4^{\prime \prime} \times 7-1 / 4^{\prime \prime}$ |  |  | $1-3 / 4^{\prime \prime} \times 9-1 / 4^{\prime \prime}$ |  |  | $1-3 / 4^{\prime \prime} \times 9-1 / 2^{\prime \prime}$ |  |  | $1-3 / 4^{\prime \prime} \times 11-1 / 4^{\prime \prime}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & \text { Live } \\ & \text { Load } \end{aligned}$ | Total Load |  | $\begin{aligned} & \text { Live } \\ & \text { Load } \end{aligned}$ | Total Load |  | $\begin{aligned} & \text { Live } \\ & \text { Load } \end{aligned}$ | Total Load |  | $\begin{aligned} & \text { Live } \\ & \text { Load } \end{aligned}$ | Total Load |  |
|  |  | Snow | Non-Snow |  | Snow | Non-Snow |  | Snow | Non-Snow |  | Snow | Non-Snow |
|  | L/240 | 115\% | 125\% | L/240 | 115\% | 125\% | L/240 | 115\% | 125\% | L/240 | 115\% | 125\% |
| 6 |  | 877 | 954 |  | 1182 | 1285 |  | 1223 | 1329 |  | 1523 | 1656 |
| 7 | 684 | 668 | 727 |  | 976 | 1062 |  | 1008 | 1097 |  | 1245 | 1354 |
| 8 | 458 | 511 | 555 |  | 810 | 881 |  | 852 | 927 |  | 1053 | 1145 |
| 9 | 321 | 403 | 425 | 668 | 639 | 695 | 724 | 672 | 731 |  | 912 | 991 |
| 10 | 234 | 309 | 309 | 487 | 517 | 562 | 527 | 544 | 591 |  | 749 | 815 |
| 11 | 176 | 231 | 231 | 366 | 426 | 464 | 396 | 448 | 488 | 658 | 618 | 672 |
| 12 | 135 | 177 | 177 | 282 | 357 | 371 | 305 | 376 | 402 | 507 | 518 | 564 |
| 13 | 106 | 138 | 138 | 221 | 291 | 291 | 240 | 315 | 315 | 399 | 441 | 480 |
| 14 | 85 | 110 | 110 | 177 | 232 | 232 | 192 | 251 | 251 | 319 | 379 | 413 |
| 15 | 69 | 89 | 89 | 144 | 187 | 187 | 156 | 203 | 203 | 259 | 330 | 340 |
| 16 | 57 | 72 | 72 | 118 | 154 | 154 | 128 | 167 | 167 | 214 | 279 | 279 |
| 17 | 47 | 60 | 60 | 99 | 127 | 127 | 107 | 138 | 138 | 178 | 232 | 232 |
| 18 | - | - | - | 83 | 106 | 106 | 90 | 115 | 115 | 150 | 194 | 194 |
| 19 | - | - | - | 71 | 90 | 90 | 76 | 97 | 97 | 127 | 164 | 164 |
| 20 | - | - | - | 60 | 76 | 76 | 65 | 83 | 83 | 109 | 140 | 140 |
| 21 | - | - | - | 52 | 65 | 65 | 57 | 71 | 71 | 94 | 120 | 120 |
| 22 | - | - | - | 45 | 56 | 56 | 49 | 61 | 61 | 82 | 104 | 104 |
| 23 | - | - | - | - | - | - | - | - | - | 72 | 90 | 90 |
| 24 | - | - | - | - | - | - | - | - | - | 63 | 78 | 78 |


| Span (ft) | $1-3 / 4^{\prime \prime} \times 11-7 / 8^{\prime \prime}$ |  |  | $1-3 / 4^{\prime \prime} \times 14^{\prime \prime}$ |  |  | $1-3 / 4^{\prime \prime} \times 16^{\prime \prime}$ |  |  | $1-3 / 4^{\prime \prime} \times 18^{\prime \prime}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Live <br> Load | Total Load |  | Live <br> Load | Total Load |  | Live Load | Total Load |  | $\begin{aligned} & \text { Live } \\ & \text { Load } \end{aligned}$ | Total Load |  |
|  |  | Snow | Non-Snow |  | Snow | Non-Snow |  | Snow | Non-Snow |  | Snow | Non-Snow |
|  | L/240 | 115\% | 125\% | L/240 | 115\% | 125\% | L/240 | 115\% | 125\% | L/240 | 115\% | 125\% |
| 6 |  | 1638 | 1782 |  | 2065 | 2245 |  | 2523 | 2743 |  | 3049 | 3315 |
| 7 |  | 1335 | 1451 |  | 1661 | 1806 |  | 2003 | 2178 |  | 2384 | 2593 |
| 8 |  | 1126 | 1224 |  | 1389 | 1510 |  | 1660 | 1805 |  | 1957 | 2128 |
| 9 |  | 973 | 1058 |  | 1193 | 1298 |  | 1417 | 1541 |  | 1659 | 1804 |
| 10 |  | 830 | 903 |  | 1046 | 1137 |  | 1236 | 1344 |  | 1439 | 1565 |
| 11 |  | 685 | 745 |  | 930 | 1012 |  | 1095 | 1191 |  | 1271 | 1382 |
| 12 | 596 | 575 | 625 |  | 781 | 850 |  | 984 | 1070 |  | 1138 | 1237 |
| 13 | 469 | 489 | 532 |  | 665 | 723 |  | 853 | 928 |  | 1029 | 1120 |
| 14 | 375 | 420 | 458 | 615 | 572 | 623 |  | 734 | 799 |  | 915 | 995 |
| 15 | 305 | 365 | 398 | 500 | 497 | 541 |  | 639 | 695 |  | 796 | 866 |
| 16 | 251 | 320 | 329 | 412 | 436 | 475 |  | 560 | 610 |  | 698 | 760 |
| 17 | 209 | 273 | 273 | 343 | 386 | 420 | 513 | 495 | 539 |  | 617 | 672 |
| 18 | 176 | 229 | 229 | 289 | 343 | 374 | 432 | 441 | 480 |  | 550 | 598 |
| 19 | 150 | 194 | 194 | 246 | 307 | 321 | 367 | 395 | 430 | 523 | 492 | 536 |
| 20 | 128 | 165 | 165 | 211 | 274 | 274 | 315 | 355 | 387 | 448 | 443 | 483 |
| 21 | 111 | 142 | 142 | 182 | 236 | 236 | 272 | 322 | 350 | 387 | 401 | 437 |
| 22 | 96 | 123 | 123 | 158 | 204 | 204 | 236 | 292 | 307 | 337 | 365 | 397 |
| 23 | 84 | 107 | 107 | 138 | 178 | 178 | 207 | 267 | 268 | 295 | 333 | 363 |
| 24 | 74 | 93 | 93 | 122 | 155 | 155 | 182 | 235 | 235 | 259 | 305 | 332 |
| 25 | 65 | 82 | 82 | 108 | 137 | 137 | 161 | 207 | 207 | 229 | 280 | 297 |
| 26 | 58 | 72 | 72 | 96 | 121 | 121 | 143 | 183 | 183 | 204 | 259 | 263 |
| 27 | 52 | 63 | 63 | 85 | 107 | 107 | 128 | 162 | 162 | 182 | 234 | 234 |
| 28 | 46 | 56 | 56 | 76 | 95 | 95 | 114 | 145 | 145 | 163 | 209 | 209 |
| 29 | - | - | - | 69 | 85 | 85 | 103 | 129 | 129 | 147 | 187 | 187 |
| 30 | - | - | - | 62 | 76 | 76 | 93 | 116 | 116 | 133 | 168 | 168 |

## NOTES:

1. Span is defined as center-to-center of bearings along the slope of the beam, and is valid for simple span and equal, multiple span conditions. For beams with a slope 2:12 or greater, the horizontal span must be multiplied by the appropriate slope adjustment factor from the table to the right.
2. These loads assume full lateral bracing of the compression edge. Full support is considered to be a maximum unbraced length of 24 ".
3. The designer must check the appropriate Total Load column and the Live Load $\mathrm{L} / 240$ column. To design for a Live Load deflection of $\mathrm{L} / 360$ or $\mathrm{L} / 480$, use the appropriate column from the Uniform Floor Load tables on page 6. Do not use a product where designated "-" without further analysis by a professional engineer.
4. The Total Load columns are limited to a deflection of $\mathrm{L} / 180$ under Total Load and do not include the effects of long term loading (creep).
5. The Total Load columns have been adjusted to account for the self-weight of the beam.
6. The Total Load columns for Snow ( $115 \%$ ) are for normal snow load designs. Check local code requirements for design snow loads and the appropriate load duration factor. Use the Total Load column from the Uniform Floor Load tables when the load duration factor is less than $115 \%$.
7. Proper bearing must be provided. Bearing length must be checked for support reactions with the table on page 2.
8. Depths of $16^{\prime \prime}$ and greater should be used with a minimum of two plies unless designed specifically as a single ply of $1-3 / 4^{"}$ with proper lateral bracing spaced at most every 24" along the length of the beam. (Example: The marriage beam for each half of a manufactured home before the units are joined.)

| SLOPE ADJUSTMENT |  |
| :---: | :---: |
| Slope | Factor |
| $2: 12$ | 1.014 |
| $3: 12$ | 1.031 |
| $4: 12$ | 1.054 |
| $5: 12$ | 1.083 |
| $6: 12$ | 1.118 |
| $7: 12$ | 1.158 |
| $8: 12$ | 1.202 |
| $9: 12$ | 1.250 |
| $10: 12$ | 1.302 |
| $11: 12$ | 1.357 |
| $12: 12$ | 1.414 |

9. The values in the table are for a single ply of $1-3 / 4^{" L}$ LVL. Double the values for a 2 -ply or $3-1 / 2^{\prime \prime}$ thickness. (Or divide design loads by 2 to use the table directly to verify each ply of a 2 -ply beam.) Triple the values for 3 -ply or $5-1 / 4^{\prime \prime}$ thickness. (Or divide design loads by 3 to use the table directly to verify each ply of a 3-ply beam.) Quadruple the values for 4 -ply or 7" thickness. (Or divide the design loads by 4 to use the table directly to verify each ply of a 4-ply beam.)
10. Side-loaded beams built up from multiple plies of LVL (e.g., supporting joists connected to the beam by hangers) may have a limited load capacity depending on the method of connecting the plies. Refer to page 13 of the Engineered Wood Product Guide for connection details and limits on side-loaded members.

## LP LVL 2650F ${ }_{\mathbf{b}}$-1.9E

LP LVL 2650 $F_{b}$ is available in:

- lengths up to $60^{\prime}$
- thicknesses of $1-1 / 2^{\prime \prime *}$ and $1-3 / 4^{\prime \prime}$
- billet thicknesses of $3-1 / 2$ ", $5-1 / 4^{\prime \prime}$ and $7^{\prime \prime}$
- available depths of $7-1 / 4$ ", $9-1 / 2$ ", $11-7 / 8^{\prime \prime}, 14$ ", 16 ", 18 ", 20 ", $22^{\prime \prime}$ and $23-7 / 8^{\prime \prime *}$.

In addition to the standard natural finish, a water-resistant coating called SiteCote ${ }^{\text {m" }}$ is available for extra weather protection during construction.

## Code Evaluation

LP Laminated Veneer Lumber has been evaluated for compliance with the major code evaluation services and many others. For the most current code reports contact your LP Engineered Wood Products distributor or visit www.Ipcorp.com.

* Contact your local distributor for availability.

LP Engineered Wood Products are manufactured at different locations in the United States and Canada. Please verify availability with the LP Engineered Wood Products distributor in your area before specifying these products.

For more information on the full line of LP Engineered Wood Products or the nearest distributor, please contact 1.800.999.9105 or e-mail customer.support@lpcorp.com.
Visit our web site at www.lpcorp.com.


[^0]:    * Deflections rounded to the nearest $1 / 16^{\text {" }}$.

